TransModeler Training 7- Model Review and Calibration

presented to

Caltrans, District 1-Eureka

presented by

Shaghayegh (Rira) Shabanian, CS



Think *Forward*

November 2018

DTA Review

Convergence:





DTA Review

Travel Time Fluctuation:

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🕅 Da	ataview1 - Histori	cal Travel Time	5														- • ×
	ID AB_Tim	e_0700 BA_Ti	me_0700 AB_Tim	e_0715 BA	_Time_0715 AB_Time	e_0730 BA_Tir	ne_0730 AB_T	ime_0745 BA_1	Time_0745 AB_1	Time_0800 BA_Ti	me_0800 AB_T	ime_0815 BA_T	ime_0815 AB_1	ime_0830 BA_T	ime_0830 AB_Tim	e_0845 BA	Time_0845 Al /
	7716	68.8	60.8	68.9	61.3	69.3	60.7	70.6	60.6	69.8	60.3	70.3	61.1	69.2	61.1	67.7	61.1
	4598	59.9		60.4		59.9		59.3		60.5		60.2		60.4		59.1	
	4640	52.8	63.5	57.2	64.0	55.6	60.8	55.4	68.4	56.1	65.4	55.4	63.4	55.8	60.8	55.9	65.8
	8065	51.3		51.3		51.3		51.3		51.3		51.3		51.3		51.3	
	7684	50.8	10.8	50.8	10.8	50.8	10.8	50.8	10.8	50.8	10.8	50.8	10.8	50.8	10.8	50.8	10.8
	8066	41.5		42.1		42.5		42.3		42.7		42.5		42.4		42.5	
	8094	42.0	43.5	42.0	43.3	41.7	43.6	42.0	44.4	42.9	43.8	42.4	44.0	41.9	43.4	41.4	43.5
	6651	39.2		39.4		39.5		39.5		39.5		39.7		39.7		39.8	
	8023	40.6	10.4	37.7	10.3	41.8	10.2	38.4	10.3	38.4	10.5	46.9	10.5	45.8	10.3	41.2	10.2
	8040	17.7	2.9	18.3	2.9	32.7	2.9	36.2	2.8	39.1	2.8	31.0	2.9	39.0	2.8	33.6	2.8
	7991	32.6	7.7	32.6	7.8	32.6	7.8	32.6	8.0	29.9	7.7	34.8	7.9	28.7	7.7	30.1	7.7
	4174	31.8	13.5	31.8	15.8	31.8	15.0	31.8	15.7	31.8	14.9	31.8	16.8	31.8	14.2	31.8	14.5
	8093	30.0		30.7		30.0		30.0		29.3		30.2		29.8		30.2	
	3850	29.5	12.5	29.5	13.5	29.5	13.8	29.5	15.0	29.5	14.7	29.5	13.3	29.4	15.4	29.4	14.1
	4691	22.3		21.8		26.3		29.0		27.2		26.4		21.9		22.9	
	4052	28.9	11.2	28.9	11.2	28.9	11.2	28.9	11.2	28.9	11.2	28.9	11.2	28.9	11.2	28.9	11.2
	8077	27.7	27.8	27.5	27.9	27.6	28.3	28.2	28.1	28.4	28.2	28.3	27.9	28.4	28.4	28.5	28.2
	4534	26.4		25.9		25.9		26.1		26.1		25.7		25.9		25.9	
	8082	14.0		17.8		18.2		24.5		24.0		26.5		24.3		21.9	
	4612	23.5	10.5	23.5	10.5	23.5	10.7	23.4	10.4	26.1	10.5	27.3	10.5	26.7	10.4	21.1	10.4
	3822	23.0	14.0	23.0	15.0	23.0	15.8	23.0	17.3	23.0	17.8	23.0	15.4	23.0	16.2	23.0	14.9
_	6934	22.2		22.3		22.3		22.3		22.3		22.4		22.3		22.4	
	4101	20.6	28.3	21.8	25.8	23.8	28.2	22.0	28.7	20.5	26.7	21.0	26.4	21.0	28.1	21.6	26.7
_	3765	16.4	25.2	23.9	26.6	26.4	26.6	21.6	38.5	24.6	38.5	18.3	38.5	16.8	38.5	21.8	38.5
	4641	21.5	27.5	20.4	30.4	23.2	22.7	21.0	21.0	22.7	24.4	24.6	22.3	27.3	27.5	25.0	24.2

Sorted the historical travel time table decreasing for 7:30 and 7:45, checked the higher travel times and their fluctuation over time. Looks reasonable.



Calibration

- Calibration is a process whereby the analyst selects the model parameters that cause the model to best reproduce field-measured local traffic operations conditions.
- A robust calibration leads to reliable future and alternative analysis
- Usually a modeler tries to calibrate:

link volume and intersection turning movements link speed and intersection delay link/intersection queue length temporal and spatial limits of bottlenecks route choice OD pattern



Interaction between Volume and Speed

- On fundamental speed-flow curve, there are always two traffic states with the same flow but different speeds. One is stable and has demand and flow lower than capacity, and the other one is unstable, has flow lower than and demand higher than the capacity.
- In presence of congestion, replicating volume without speed is useless. But simultaneous replication of volume and speed guarantees replication of the demand.
- Calibrating speed and volume is usually iterative, and adjusting one, will affect the other one.



Calibration approach

- Since calibration of demand, route choice, and network parameters is an iterative procedure, it is recommended to approach calibration through these steps to minimize the number of iterations between these components:
- 1. Error checking
- 2. Setting capacity parameters
- 3. Calibration of demand and route choice
- 4. Fine tuning for delay and speeds

Following these steps is particularly useful in congested and complex networks.





- Network connectivity (track a single vehicle along a route, assure there is no superimposed link, node, run the model with low demand and make sure all vehicles are assigned)
- Facility type, number of lanes (color coded maps)
- Intersection geometry, conflicts & priorities
- Speed limits, reduced speed areas
- Signal timings
- Demand (total generated and attracted, OD Desire Lines, aggregated vehicle composition, vehicle performance specification)
- Run model with 40%-50% demand, any congestion?
- trace single vehicle through network and see if there is any location it unexpectedly slows down.
- Run and watch animation with 20% and then 50% demand



Set Capacity Parameters*

- Uninterrupted flow: Capacity-metered volume (locations where queues persist for at least 15 min)
- Signalized Intersections: Identify the approach legs that frequently have queues of at least 10 vehicles per lane and measure the saturation flow rate per hour per lane

$$c = s \frac{g}{C}$$
 (Equation 2)

where:

c = capacity (vehicles per hour (veh/h))

s = saturation flow (veh/h per green phase)

g = effective green time

C = cycle length

Several measurements of maximum flow rates should be made in the field and averaged.



Set Capacity Parameters

Freeway Facilities:

Mean following headway.

Driver reaction time.

Critical gap for lane changing.

Minimum separation under stop-and-go conditions.

Signalized Intersections:

Startup lost time.

Queue discharge headway.

Gap acceptance for unprotected left turns.



Driver behavior

- Aggressiveness
- Reaction time
- Desired speed
- Acceptable critical gap
- Cooperation
- Awareness (familiarity with the road and traffic condition)
- Compliance



Calibrating Demand and Route Choice

This is an iterative effort

- First, check if for screenlines, if total model volume matches total observed counts. If they match, the mismatch of volume/count on individual route can be fixed by improving the route travel time and route choice parameter, if not, it means the demand needs to be adjusted*.
- The analyst must adjust the input demand as necessary to create a queue upstream of the target section to be calibrated so that the model will report the maximum possible flow rate through the bottleneck.
- If the model initially shows congested bottlenecks at locations that DO NOT exist in the field, it will be necessary to temporarily increase the capacity at those false bottlenecks (using temporary link-specific headway adjustments). These temporary adjustments are then removed during the fine-tuning phase.



Calibrating Demand and Route Choice-cont

- Once the analyst is satisfied that the model reproduces as closely as possible the field-measured capacities, the next step is to then calibrate the route choice parameters in the model to better match the observed flows. The temporary demand adjustments used in the previous capacity calibration step are reversed. The model-predicted volumes are then compared to the field counts and the analyst adjusts the route choice algorithm parameters until the best volume fit is achieved.
- usually involve weightings placed on the actual cost and travel time for each route.



Parameter selection

- Global vs local, for having a robust model, it is better to first, and as much as possible to change global parameters tat affect the whole model, and adjust the local parameters if defendable.
- Keep it manageable: The analyst should attempt to keep the set of adjustable parameters as small as possible to minimize the effort required to calibrate them. Whenever practical, the analyst should use observed field data to reflect local conditions. This observed data will serve as the nonadjustable values for certain calibration parameters, thus leaving the set of adjustable parameters to a minimum. However, the tradeoff is that more parameters allow the analyst more degrees of freedom to better fit the calibrated model to the specific location.
- Avoid overfitting: a model may perfectly match the current situation, but could poorly predict the future conditions, response unreasonably to small change in network or demand and will not be reliable for alternative analysis.



Caltrans Calibration Recommendation

Criteria & Measures	Acceptability Targets
Hourly Flows, Model vs. Observed Individual Link Flows Within 15%, for 700 vph < Flow <2700 vph Within 100 vph, for Flow < 700 vph Within 400 vph, for Flow > 2700 vph	> 85% of cases > 85% of cases > 85% of cases
Total Link Flows Within 5% GEH Statistic – Individual Link Flows GEH < 5 GEH Statistic – Total Link Flows GEH < 4	All Accepting Links > 85% of cases All Accepting Links
Travel Times, Model vs. Observed Journey Times Network Within 15% (or one minute, if higher)	> 85% of cases
Visual Audits Individual Link Speeds Visually acceptable Speed-Flow relationship Bottlenecks Visually acceptable Queuing	To analyst's satisfaction To analyst's satisfaction

Source: FREEWAY SYSTEM OPERATIONAL ASSESSMENT, Technical Report I-33, Paramics Calibration & Validation Guidelines, DRAFT, Wisconsin Department Of Transportation, District 2, June 2002



- Volume: Manually and slightly adjusting trip tables (already a result of ODME), only for OD pairs that contribute significantly on link volumes that are significantly different from observed count. This adjustment should be accompanied by local knowledge and judgment.
- TransModeler provides a tool that determine OD pairs that would potentially pass thorough a certain link, or turning movement
- Make sure in the project setting, the output folder has the address of your desirable run
- ➢ Go to Demand → Critical Link Tools* turning movement 8016-842046-7740 under-estimated for 8 to 9 a.m. and we like to increase the trips that contribute to this turning movement:





CAMBRIDGE SYSTEMATICS

Review major contributors to this turning movement by clicking on those with highest number of trips:



Make a copy of the estimated OD and call it Adjusted OD

Open the adjusted OD file and click on / to edit the matrix



You can apply a decreasing or increasing factor to all matrices or individual time periods that contribute on this turning:

Adjust Trip Matrix									
Settings									
Adjust Estimated OD Matrix	~								
By Multiplying 0730 0745 0800 0815 0830	^								
Total	~								
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Based on Critical Link Matrix									
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Profile (4 Matrices)									
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Trips in selected matrices before adjustment 70									
Trips on the critical link(s) after adjustment 105	_								
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The model has generally smaller travel time compared to the observed travel time. The free flow speed can be adjusted to improve the route travel time replication.



Turning Movement Replication - AM



➢ GEH<5: 7 to 8: 71%, 8 to 9: 71%</p>

→ GEH<10: 7 to 8: 96%, 8 to 9: 94%</p>



Turning Movement Replication - PM



➢ GEH<5: 4 to 5: 66%, 5 to 6: 59%</p>

→ GEH<10: 4 to 5: 89%, 5 to 6: 89%</p>



Segment Volume Replication- AM



→ GEH<5: 7 to 8: 94%, 8 to 9: 94%</p>



Segment Volume Replication- PM



➤ GEH<5: 4 to 5: 80%, 5 to 6: 72%</p>

→ GEH<5: 4 to 5: 95%, 5 to 6: 92%</p>



Route Travel Time Replication-AM

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						Outer Reach	1 B2 103	
			71	TO 8 AM		1	With St 7th St	0.5
	Route	min Obs	Observed	Max Obs	Base	% Obs_Base	W705 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
Total		266	294	349	318	8%	S W Washington St.	
on 4 st		75	77	85	84	9%		
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_		280	340	400	340	0%	W Simpson St. Simpson St	
on 5th st		81	102	111	93	-9%	a ri Cedar St. Cedar St.	
On Broadway Wabash to 5 st		na	na	na	132		#146.5 (F 146.5 146.5	
SE	3	357	401	401	419	4%		
NE	3	399	465	465	413	-11%	156 St 156 St 156 St 156 St 156 St	
		382	425	478	463	9%	B W Chards St. Church St. Viewer Art	22
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Total on 4 st On Koster Wabsh to 4 st on 5th st	Route	371 min Obs 266 75 na 280 81	371 8 t Observed 307 77 na 292 102	461 o 9 AM Max Obs 349 85 na 400 111	474 Base 330 86 134 335 96	28% % Obs_Base 7% 12% 15% -6%	Convert St. Convert St. Conve	g Eu
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Pearl St. Rarill St.

Route Travel Time Replication-PM

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	Route	min Obs	Observed	Max Obs	Base	% Obs_Base			101	617	n SL
Total		339	395	545	386	-2%	and pr	80	R dia	5 5 7	th S
on 4 st		82	92	108	89	-3%	an se	W 6th St	7th St.	1.0	20.5
On Koster Wabsh to 4 st		na	na	na	162			WI	C L MR S S		G
		319	379	446	364	-4%	W Washington St	Surren St	den		
on 5th st		79	103	123	105	2%	No.	4 H	N Grant St		
On Broadway Wabash to 5 st		na	na	na	146			A Clark St.	Cat St		
SB		421	533	592	489	-8%		W Seep	son St. Simpson St. Hillsdal	20	L
NB		424	459	524	446	-3%	No.	W Cedar St	Cedar St		Ł
		446	498	533	499	0%	45.2 2	145.52	14th St		
		497	529	563	490	-7%			104.0		
					C	ostco Wholesale	horts	W Church S	Church R	Andread States and Sta	F
			5 T	O 6 PM			W Walkash Ave		5.00	and the	
	Route	min Obs	Observed	Max Obs	Base	% Obs_Base		Pine S Summ Union Albee	Lowel C Sa E St E St Califor	🖁 🚦 Eureka Hig	S
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on 4 st		82	92	108	90	-2%		me 92	Numbol	a 31	H
On Koster Wabsh to 4 st		na	na	na	166				Trinity St		
		319	379	446	371	-2%	l Alban	z	Huttoon St		
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Alternative Model Development

- Once a model is reasonably calibrated, alternative networks can be built.
- Each alternative will have a separate folder, .smp, underlying .dbd, and other inputs.
- While modifying the network for alternative analysis please check:
- I. When deleting a link, did that link include count? Do you need to adjust the turning movement bin file?
- II. When adding a link, fill the free flow speed at segment layer, and function classification at link layer
- III. How the change will affect the signal timing? For some locations new signal timing is needed. You can fine tune it by watching the simulation.
- IV. Be mindful of how to change centroid connectors as a result of the network modification.

